

GEOTECHNICAL INVESTIGATION AND ACID SULFATE SOIL ASSESSMENT

FOR

NSW LAND & HOUSING CORPORATION

2-8 Glen Avenue, Northmead, New South Wales (BGX6Q)

Report No: 20/3472

Project No: 30790/4406D-G

October 2020

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DRAWING NO. 20/3472 – BOREHOLE AND PENETROMETER LOCATIONS

NOTES RELATING TO GEOTECHNICAL REPORTS

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1. INTRODUCTION

This report presents the results of a combined Geotechnical Investigation and Acid Sulfate Soil (ASS) assessment carried out by STS Geotechnics Pty Limited (STS) for a proposed new residential development to be constructed at 2-8 Glen Avenue, Northmead. At the time of writing this report STS were not provided with architectural drawings for the project, however we understand the development will typically comprise the construction of single and double storey residential buildings. The development will not include basement levels. We understand that the site is located within a Class 5 Acid Sulfate Soils area and therefore Council requires an assessment to be undertaken.

The purpose of the investigation was to:

- assess the subsurface conditions over the site,
- provide a Site Classification to AS2870,
- provide recommendations regarding the appropriate foundation system for the site including design parameters,
- comment on soil aggressiveness to buried steel and concrete,
- undertake an ASS assessment, and
- determine if an ASS Management Plan is required.

The investigation was undertaken at the request of NSW Land and Housing Corporation.

Our scope of work did not include a contamination assessment.

2. NATURE OF THE INVESTIGATION

2.1. Fieldwork

The fieldwork consisted of drilling six (6) boreholes numbered BH1 to BH6, inclusive, at the locations shown on Drawing No. 20/3472. Restricted site access dictated the borehole locations. The boreholes were drilled using an Edson RP70 drilling rig owned and operated by STS. Soils and weathered rock were drilled using rotary solid flight augers. Soil strengths were determined by undertaking Dynamic Cone Penetrometer (DCP) tests at each borehole location. Drilling operations were undertaken by one of STS's senior geologists who also logged the subsurface conditions encountered.

The subsurface conditions observed are recorded on the borehole logs given in Appendix A. An explanation of the terms used on the logs is also given in Appendix A. Notes relating to geotechnical reports are also attached.

2.2. Laboratory Testing

In order to assist with determining the site classification, a shrink swell index test was carried out on a representative sample retrieved from the site.

In order to assess the soils for their aggressiveness, a selected representative soil sample was tested to determine the following:

- pH,
- Sulphate content (SO₄),
- Chloride content (CL), and
- Electrical Conductivity (EC)

Based on field observations, four soil samples were also selected for laboratory analysis for the Acid Sulfate Soils assessment. The samples were dispatched to Australian Laboratory Services (ALS) for analysis using the Suspension Peroxide Oxidation Combined Acidity and Sulphate (SPOCAS) method. The method allows both a measure of the existing and potential acidity.

Detailed test reports are given in Appendix B.

3. GEOLOGY AND SITE CONDITIONS

The Penrith geological series sheet at a scale of 1:100,000 shows Triassic Age Hawkesbury Sandstone underlies the site. Rocks within this formation comprise medium to coarse grained quartz sandstone.

The site is trapezoidal in shape with an area of approximately 2895 m². At the time of the fieldwork, fibro houses were present. Site vegetation comprised grass, trees and shrubs.

The ground surface falls approximately 3 metres to the north-west. Single and double storey residential dwellings are present in the nearby properties.

4. SUBSURFACE CONDITIONS

When assessing the subsurface conditions across a site from a limited number of boreholes, there is the possibility that variations may occur between test locations. The data derived from the site investigation programme are extrapolated across the site to form a geological model and an engineering opinion is rendered about overall subsurface conditions and their likely behaviour regarding the proposed development. The actual condition at the site may differ from those

inferred, since no subsurface exploration programme, no matter how comprehensive, can reveal all subsurface details and anomalies.

The subsurface conditions generally consist of topsoil and fill overlying silty sandy clays, gravelly sandy clays, silty clays and weathered sandstone. Topsoil and fill were encountered to approximate depths of 0.2 to 0.6 metres. Natural silty sandy clays, gravelly sandy clays and silty clays underlie the topsoil and fill to approximate depths of 1.0 to 2.4 metres. The consistency of these materials range between firm to stiff and hard. Weathered sandstone bedrock underlies the natural soils to the auger refusal depths of 0.7 to 2.9 metres.

Groundwater was not observed during drilling of the boreholes.

5. GEOTECHNICAL DISCUSSION

5.1. Site Classification to AS2870

The classification has been prepared in accordance with the guidelines set out in the “Residential Slabs and Footings” Code, AS2870 – 2011.

The sample collected from BH2 was unsuitable for shrink swell testing. Experience has shown that at times, the shrink swell index can be estimated by dividing the soil Plasticity Index (PI) by a factor of 10. The soil tested was non plastic which infers minimal shrink swell potential.

Undisturbed samples were obtained to determine their shrink swell index.

The results are summarised in Table 5.1

Table 5.1 – Shrink Swell Summary

Location	Depth (m)	Material Description	Shrink/Swell Index (% per ΔpF)
BH3	0.5-0.7	Silty sandy clay	1.0
BH4	0.5-0.7	Silty sandy clay	1.6
BH6	0.4-0.6	Silty gravelly clay	2.1

Because of the trees and dwellings present, abnormal moisture conditions (AMC) prevail at the site. (Refer to Section 1.3.3 of AS2870).

Because of the AMC and fill present, the site is classified a *problem site (P)*. However, provided the recommendations given below are adopted and the footings bear in the underlying natural soils, the site may be reclassified *moderately reactive (M)*.

5.2. Foundation Design

The existing fill and topsoil materials should not be relied upon for foundation support. Footings that bear in the firm to stiff natural clayey soils may be proportioned using an allowable bearing pressure of 100 kPa. The minimum depth of founding must comply with the requirements of AS2870-2011. In order to overcome the presence of the trees along the site boundaries, the foundations should be designed in accordance with the procedures given in Appendices H and CH of AS2870-2011. Tree information is attached.

Should a higher bearing pressure be required then piles can be used. Piles founded in weathered sandstone may be proportioned using an allowable bearing pressure of 800 kPa. An allowable adhesion of 80 kPa may be adopted for the portion of the shaft within the weathered shale. When piles are founded in shale the adhesion within the overlying soils must be ignored.

In order to ensure the bearing values given can be achieved, care should be taken to ensure that the base of excavations are free of all loose material prior to concreting. It is recommended that all footing excavations be protected with a layer of blinding concrete as soon as possible, preferably immediately after excavating, cleaning, inspection and approval. The possible presence of groundwater needs to be considered when drilling piers and pouring concrete.

5.3. Soil Aggressiveness

The aggressiveness or erosion potential of an environment in building materials, particularly concrete and steel is dependent on the levels of soil pH and the types of salts present, generally sulphates and chlorides. In order to determine the degree of aggressiveness, the test values obtained are compared to Tables 6.4.2 (C) and 6.5.2 (C) in AS2159 – 2009 Piling – Design and Installation. The test results are summarised in Table 5.2 below.

Table 5.2– Soil Aggressiveness Summary Table

Sample No.	Location	Depth (m)	pH	Sulfate (mg/kg)	Chloride (mg/kg)	Electrical Conductivity (dS/m)	
						EC _{1:5}	EC _e
S1	BH1	0.2	6.2	10	10	0.028	0.3
S2	BH2	0.3	5.2	20	<10	0.030	0.3
S3	BH3	0.4	7.6	6790	<10	1.970	17.7
S4	BH4	0.4	7.4	<10	<100	0.033	0.3

The soils on the site are cohesive in nature. Therefore, the soil conditions B are considered appropriate.

A review of the durability aspects indicates that:

- pH : minimum value of 5.2
- SO₄ : maximum value of 6970 mg/kg (ppm) > 5000 ppm
- Cl : maximum value of 100 mg/kg (ppm) < 5000 ppm
- EC_e : maximum value of 17.7 dS/m

In accordance with AS2159-2009, the exposure classification for the onsite soils is non-aggressive to steel and mildly aggressive to concrete. The soils are classified as B2 in accordance with AS2870-2011. This classification is due to the EC_e value for BH3. It should be noted this is fill material and not representative of the onsite natural soils. The natural soils are classified A2.

Reference to DLWC (2002) "Site Investigations for Urban Salinity" indicates that the EC_e value of 0.3 dS/m for the natural soils is consistent with the presence of non-saline soils. The fill material is highly saline.

6. ACID SULFATE SOIL ASSESSMENT

6.1. Introduction

ASS are the common name given to sediments and soils containing iron sulfides which, when exposed to oxygen generate sulfuric acid. Natural processes formed the majority of acid sulfate sediments when certain conditions existed in the Holocene geological period (the last 10,000 years). Formation conditions require the presence of iron-rich sediments, sulfate (usually from seawater), removal of reaction products such as bicarbonate, the presence of sulfate reducing bacteria and a plentiful supply of organic matter. It should be noted that these conditions exist in mangroves, salt marsh vegetation or tidal areas, and at the bottom of coastal rivers and lakes.

The relatively specific conditions under which acid sulfate soils are formed usually limit their occurrence to low lying parts of coastal floodplains, rivers and creeks. This includes areas with saline or brackish water such as deltas, coastal flats, backswamps and seasonal or permanent freshwater swamps that were formerly brackish. Due to flooding and stormwater erosion, these sulfidic sediments may continue to be re-distributed through the sands and sediments of the estuarine floodplain region. Sulfidic sediment may be found at any depth in suitable coastal sediments – usually beneath the water table.

Any lowering in the water table that covers and protects potential ASS will result in their aeration and the exposure of iron sulfide sediments to oxygen. The lowering in the water table can occur naturally due to seasonal fluctuations and drought or any human intervention, when carrying out any excavations during site development. Potential ASS can also be exposed to air during physical disturbance with the material at the disturbance face, as well as the extracted material,

both potentially being oxidised. The oxidation of iron sulfide sediments in potential ASS results in ASS soils.

Successful management of areas with ASS is possible but must take into account the specific nature of the site and the environmental consequences of development. While it is preferable that sites exhibiting acid sulfate characteristics not be disturbed, management techniques have been devised to minimise and manage impacts in certain circumstances.

When works involving the disturbance of soil or the change of groundwater levels are proposed in coastal areas, a preliminary assessment should be undertaken to determine whether acid sulfate soils are present and if the proposed works are likely to disturb these soils.

6.2. Presence of ASS

Reference to the Prospect-Parramatta ASS Risk Map indicates the property is within an area where there are no known occurrences of Acid Sulfate Soils. It should be noted that maps are a guide only.

The following geomorphic or site criteria are normally used to determine if acid sulfate soils are likely to be present:

- sediments of recent geological age (Holocene)
- soil horizons less than 5 in AHD
- marine or estuarine sediments and tidal lakes
- in coastal wetlands or back swamp areas

6.3. Assessment

In order to assess the significance of the ASS potential, the laboratory results carried out were compared to action criteria contained in ASSM (1998) summarised in Table 6.1. The action criteria trigger the need to prepare an ASSMP and are based on the percentage of oxidisable sulphur (or equivalent TPA and TSA) for broad categories of soil types. Works in soils that exceed these action criteria must prepare a management plan and obtain development consent.

As the soils encountered on the site primarily consisted of clays, the fine texture grade criteria are the most appropriate and have been adopted for this assessment.

Table 6.1 – ASS Action Criteria

Type of material		Action Criteria if 1-1000 tonnes ASS disturbed		Action Criteria if more than 1000 tonnes ASS disturbed	
Texture Range (McDonald et al 1990)	Approx. clay content (%<0.02mm)	Sulphur Trail %S oxidisable (oven dry basis) eg S_{TOS} or S_{POS}	Acid Trail Mol H^+ /tonne (oven dry basis) eg TPA or TSA_s	Sulphur Trail %S oxidisable (oven dry basis) eg S_{TOS} or S_{POS}	Acid Trail Mol H^+ /tonne (oven dry basis) eg TPA or TSA_s
Coarse Texture (CT) Sands to loamy sands	≥ 5	0.03	18	0.03	18
Medium Texture (MT) Sandy loams to light clays	5-50	0.06	36	0.03	18
Fine Texture (FT) Medium to heavy clays and silty clays	≥ 40	0.1	62	0.03	18

The laboratory test results are summarised in relation to the action criteria in Table 6.2.

Table 6.2 – SPOCAS TEST RESULTS SUMMARY

Analysis	Unit	LOR	ASS1 BH1 @ 0.3m	ASS2 BH2 @ 0.5m	ASS3 BH2 @ 0.7m	ASS4 BH3 @ 0.4m	ASS5 BH3 @ 0.6m	ASS6 BH3 @ 1.0m	Action Criteria ¹ <1000 tonnes disturbed
pH before Oxidation	NA	0.1	5.4	4.7	4.9	7.9	4.9	4.2	-
pH after Oxidation	NA	0.1	3.5	4.3	4.6	6.8	4.7	4.4	<3 (high risk)
S (POS)	%	0.02	<0.02	0.022	<0.02	<0.02	<0.02	0.026	0.1
TPA	mole/tonne	2	98	73	45	<2	48	110	62
TSA	Mole/tonne	2	89	40	22	<2	25	27	62

¹ = ASSMAC (1998)

Action Criteria Exceeded

The results of the soil sample analyses are compared to the above criteria in Table 6.1, and the analytical laboratory reports for the testing performed are provided in Appendix B.

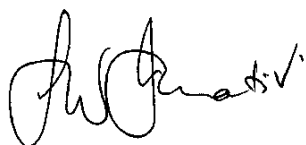
The results show that the peroxide oxidisable sulfur (POS) percentages are less than the action criteria value. The titratable peroxide acidity (TPA) concentrations measured in samples ASS1, ASS2 and ASS6 are above the 'Acid Trail' criterion of 62 mol H⁺/tonne. However, because the "Sulfur Trail" is below the detection limit, the "acid trail" recorded in these samples is due to something other than sulfur, which means that ASS is not present.

Based on the above an ASS Management Plan will not be required provided onsite dewatering does not lower the groundwater level outside the site.

7. FINAL COMMENTS

During construction, should the subsurface conditions vary from those inferred above, we would be contacted to determine if any changes should be made to our recommendations.

The exposed bearing surfaces for footings should be inspected by a geotechnical engineer to ensure the allowable pressure given has been achieved.



Laurie Ihnativ
Senior Geotechnical Engineer
STS Geotechnics Pty Limited



STS Geotechnics Pty. Ltd.

Scale: Unknown

Date: October 2020

Client: NSW LAND & HOUSING CORPORATION

**GEOTECHNICAL INVESTIGATION
2-8 GLEN AVENUE, NORTHMEAD
BOREHOLE AND PENETROMETER LOCATIONS**

Project No.
30790/4406D-G

Drawing No: 20/3472

NOTES RELATING TO GEOTECHNICAL REPORTS

Introduction

These notes have been provided to outline the methodology and limitations inherent in geotechnical reporting. The issues discussed are not relevant to all reports and further advice should be sought if there are any queries regarding any advice or report.

When copies of reports are made, they should be reproduced in full.

Geotechnical Reports

Geotechnical reports are prepared by qualified personnel on the information supplied or obtained and are based on current engineering standards of interpretation and analysis.

Information may be gained from limited subsurface testing, surface observations, previous work and is supplemented by knowledge of the local geology and experience of the range of properties that may be exhibited by the materials present. For this reason, geotechnical reports should be regarded as interpretative rather than factual documents, limited to some extent by the scope of information on which they rely.

Where the report has been prepared for a specific purpose (eg. design of a three-storey building), the information and interpretation may not be appropriate if the design is changed (eg. a twenty storey building). In such cases, the report and the sufficiency of the existing work should be reviewed by STS Geotechnics Pty Limited in the light of the new proposal.

Every care is taken with the report content, however, it is not always possible to anticipate or assume responsibility for the following conditions:

- Unexpected variations in ground conditions. The potential for this depends on the amount of investigative work undertaken.
- Changes in policy or interpretation by statutory authorities.
- The actions of contractors responding to commercial pressures.

If these occur, STS Geotechnics Pty Limited would be pleased to resolve the matter through further investigation, analysis or advice.

Unforeseen Conditions

Should conditions encountered on site differ markedly from those anticipated from the information contained in the report, STS

Geotechnics Pty Limited should be notified immediately. Early identification of site anomalies generally results in any problems being more readily resolved and allows re-interpretation and assessment of the implications for future work.

Subsurface Information

Logs of a borehole, recovered core, test pit, excavated face or cone penetration test are an engineering and/or geological interpretation of the subsurface conditions. The reliability of the logged information depends on the drilling/testing method, sampling and/or observation spacings and the ground conditions. It is not always possible or economic to obtain continuous high quality data. It should also be recognised that the volume or material observed or tested is only a fraction of the total subsurface profile.

Interpretation of subsurface information and application to design and construction must take into consideration the spacing of the test locations, the frequency of observations and testing, and the possibility that geological boundaries may vary between observation points.

Groundwater observations and measurements outside of specially designed and constructed piezometers should be treated with care for the following reasons:

- In low permeability soils groundwater may not seep into an excavation or bore in the short time it is left open.
- A localised perched water table may not represent the true water table.
- Groundwater levels vary according to rainfall events or season.
- Some drilling and testing procedures mask or prevent groundwater inflow.

The installation of piezometers and long term monitoring of groundwater levels may be required to adequately identify groundwater conditions.

Supply of Geotechnical Information or Tendering Purposes

It is recommended tenderers are provided with as much geological and geotechnical information that is available and that where there are uncertainties regarding the ground conditions, prospective tenders should be provided with comments discussing the range of likely conditions in addition to the investigation data.

APPENDIX A – BOREHOLE LOGS AND EXPLANATION SHEETS

Client: NSW Land & Housing Corporation		Project / STS No. 30790/4406D-G		BOREHOLE NO.: BH 1		
Project: 2-8 Glen Avenue, Northmead		Date: September 24, 2020		Sheet 1 of 1		
Location: Refer to Drawing No. 20/3472		Logged: JK Checked By: LI				
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S1 @ 0.2 m ASS1 @ 0.3 m	0.5	FILL: SILTY SANDY CLAY: dark brown, fine grained, low plasticity, trace of sandstone gravel	CL	FIRM	M
			WEATHERED SANDSTONE: orange brown with light grey, fine to medium grained		EXTREMELY LOW STRENGTH	D
		1.0	AUGER REFUSAL AT 0.7 M ON WEATHERED SANDSTONE			
		1.5				
		2.0				
		2.5				
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample				Contractor: STS Equipment: Edson RP70 Hole Diameter (mm): 100		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols				Angle from Vertical (°): 0 Drill Bit: Spiral		

Client: NSW Land & Housing Corporation			Project / STS No. 30790/4406D-G		BOREHOLE NO.: BH 2	
Project: 2-8 Glen Avenue, Northmead			Date: September 24, 2020			
Location: Refer to Drawing No. 20/3472			Logged: JK Checked By: LI		Sheet 1 of 1	
W A T E R L E V E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S2 @ 0.3 m		SILTY SANDY CLAY: dark brown, fine grained, low plasticity	CL	FIRM TO STIFF	M
			SILTY SANDY CLAY: orange brown, fine grained, low plasticity, trace of sandstone gravel	CL	STIFF	M
	ASS2 @ 0.2 m	0.5				
	D1 0.6-0.8 m		GRAVELLY SANDY CLAY: red brown with light grey, fine to medium grained, low plasticity, some sandstone gravel	CL	STIFF	M-D
	ASS3 @ 0.7 m					
		1.0	WEATHERED SANDSTONE: red brown with light grey, fine to medium grained, clay seams		EXTREMELY LOW STRENGTH	D
			AUGER REFUSAL AT 1.3 M ON WEATHERED SANDSTONE			
		1.5				
		2.0				
		2.5				
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample				Contractor: STS Equipment: Edson RP70 Hole Diameter (mm): 100 Angle from Vertical (°): 0 Drill Bit: Spiral		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols						

Client: NSW Land & Housing Corporation		Project / STS No. 30790/4406D-G		BOREHOLE NO.: BH 3		
Project: 2-8 Glen Avenue, Northmead		Date: September 24, 2020				
Location: Refer to Drawing No. 20/3472		Logged: JK Checked By: LI		Sheet 1 of 1		
W A T E R L E V E L	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
	S4 @ 0.4 m		FILL: SILTY CLAY: dark brown with dark grey and light grey, medium plasticity, trace of gravel, brick	CL	FIRM TO STIFF	M
	ASS4 @ 0.4 m					
	U50 0.5-0.7 m	0.5				
	ASS5 @ 0.6 m		SILTY CLAY: orange brown with light grey, medium to high plasticity, trace of fine sand, trace of sandstone gravel	CL/CH	STIFF BECOMING STIFF TO VERY STIFF	M
	ASS6 @ 1.0 m					
		1.0	SILTY CLAY: light grey with orange brown and red brown, medium to high plasticity, trace of fine sand	CL/CH	STIFF TO VERY STIFF BECOMING VERY STIFF	M
		1.5				
		2.0	SILTY SANDY CLAY: light grey with red brown and orange brown, fine to medium grained, trace of sandstone gravel	CL	VERY STIFF	M-D
		2.5	WEATHERED SANDSTONE: light grey with orange brown, fine to medium grained, clay seams		EXTREMELY LOW STRENGTH	D
			AUGER REFUSAL AT 2.9 M ON WEATHERED SANDSTONE			
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample				Contractor: STS Equipment: Edson RP70 Hole Diameter (mm): 100 Angle from Vertical (°): 0 Drill Bit: Spiral		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols						

Form: I1-2

Client: NSW Land & Housing Corporation		Project / STS No. 30790/4406D-G		BOREHOLE NO.: BH 5		
Project: 2-8 Glen Avenue, Northmead		Date: September 24, 2020		Sheet 1 of 1		
Location: Refer to Drawing No. 20/3472		Logged: JK Checked By: LI				
W A T T A E B R L E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
			TOPSOIL: SILTY CLAY: dark brown, medium plasticity	CL	STIFF	M
		0.5	SILTY CLAY: orange brown with light grey, medium to high plasticity	CL/CH	STIFF	M
		1.0	SILTY CLAY: light grey with orange brown and red brown, medium to high plasticity, trace of fine sand	CL/CH	VERY STIFF	M
		1.5	WEATHERED SANDSTONE: light grey with red brown, fine to medium grained		EXTREMELY LOW STRENGTH	D
		2.0	AUGER REFUSAL AT 1.7 M ON WEATHERED SANDSTONE			
		2.5				
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample				Contractor: STS Equipment: Edson RP70 Hole Diameter (mm): 100		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols				Angle from Vertical (°): 0 Drill Bit: Spiral		

Client: NSW Land & Housing Corporation		Project / STS No. 30790/4406D-G		BOREHOLE NO.: BH 6		
Project: 2-8 Glen Avenue, Northmead		Date: September 24, 2020				
Location: Refer to Drawing No. 20/3472		Logged: JK Checked By: LI		Sheet 1 of 1		
W A T E R L E V E	S A M P L E S	DEPTH (m)	DESCRIPTION OF DRILLED PRODUCT (Soil type, colour, grain size, plasticity, minor components, observations)	S Y M B O L	CONSISTENCY (cohesive soils) or RELATIVE DENSITY (sands and gravels)	M O I S T U R E
			TOPSOIL: SILTY CLAY: dark brown, medium plasticity	CL	FIRM TO STIFF	M
	U50 0.4-0.6 m		SILTY CLAY: orange brown with light grey, medium to high plasticity, trace of gravel, trace of fine sand	CL/CH	FIRM TO STIFF	M
		0.5			STIFF	
		1.0	SILTY CLAY: light grey with orange brown and red brown, medium to high plasticity	CL/CH	VERY STIFF	M
		1.5				
		2.0	WEATHERED SANDSTONE: light grey with orange brown, fine to medium grained, clay seams		EXTREMELY LOW STRENGTH	D
		2.5	AUGER REFUSAL AT 2.2 M ON WEATHERED SANDSTONE			
D - disturbed sample U - undisturbed tube sample B - bulk sample WT - level of water table or free water N - Standard Penetration Test (SPT) S - jar sample				Contractor: STS Equipment: Edson RP70 Hole Diameter (mm): 100 Angle from Vertical (°): 0 Drill Bit: Spiral		
NOTES: See explanation sheets for meaning of all descriptive terms and symbols						

Dynamic Cone Penetrometer Test Report

Project: 2-8 GLEN AVENUE, NORTHMEAD

Project No.: 30790/4406D-G

Client: NSW LAND & HOUSING CORPORATION

Report No.: 20/3472

Address: 31-39 Macquarie Street, Parramatta

Report Date: 8/10/2020

Test Method: AS 1289.6.3.2

Page: 1 of 1



Accredited for compliance with ISO/IEC
17025 - Testing
The results of the tests, calibrations and/or
measurements included in this document are
traceable to Australian/national standards
NATA Accreditation Number 2750

Site No.	P1	P2	P3	P4	P5	P6
Location	Refer to Drawing No. 20/3472	Refer to Drawing No. 20/3472	Refer to Drawing No. 20/3472	Refer to Drawing No. 20/3472	Refer to Drawing No. 20/3472	Refer to Drawing No. 20/3472
Date Tested	24/9/2020	24/9/2020	24/9/2020	24/9/2020	24/9/2020	24/9/2020
Starting Level	Surface Level	Surface Level	Surface Level	Surface Level	Surface Level	Surface Level
Depth (m)	Penetration Resistance (blows / 150mm)					
0.00 - 0.15	1	1	2	1	1	1
0.15 - 0.30	2	3	4	2	4	4
0.30 - 0.45	2	5	6	5	5	3
0.45 - 0.60	22	5	4	5	6	4
0.60 - 0.75	Refusal	8	5	6	5	5
0.75 - 0.90		5	6	10	5	5
0.90 - 1.05		22	7	22	8	8
1.05 - 1.20		Refusal	8	Refusal	12	10
1.20 - 1.35			7		16	10
1.35 - 1.50			7		22	9
1.50 - 1.65			9		Refusal	12
1.65 - 1.80			15			22
1.80 - 1.95			22			Refusal
1.95 - 2.10			Refusal			
2.10 - 2.25						
2.25 - 2.40						
2.40 - 2.55						
2.55 - 2.70						
2.70 - 2.85						
2.85 - 3.00						
3.00 - 3.15						
3.15 - 3.30						
3.30 - 3.45						
3.45 - 3.60						
3.60 - 3.75						

Remarks: * Pre drilled prior to testing

Approved Signatory.....



Technician: JK

Orlando Mendoza - Laboratory Manager

E1. CLASSIFICATION OF SOILS

E1.1 Soil Classification and the Unified System

An assessment of the site conditions usually includes an appraisal of the data available by combining values of engineering properties obtained by the site investigation with descriptions, from visual observation of the materials present on site.

The system used by STS Geotechnics Pty Ltd (STS) in the identification of soil is the Unified Soil Classification system (USC) which was developed by the US Army Corps of Engineers during World War II and has since gained international acceptance and has been adopted in its metricated form by the Standards Association of Australia.

The Australian Site Investigation Code (AS1726-1981, Appendix D) recommends that the description of a soil includes the USC group symbols which are an integral component of the system.

The soil description should contain the following information in order:

Soil composition

- SOIL NAME and USC classification symbol (IN BLOCK LETTERS)
- plasticity or particle characteristics
- colour
- secondary and minor constituents (name estimated proportion, plasticity or particle characteristics, colour)

Soil condition

- moisture condition
- consistency or density index

Soil structure

- structure (zoning, defects, cementing)

Soil origin

interpretation based on observation eg FILL, TOPSOIL, RESIDUAL, ALLUVIUM.

E1.2 Soil Composition

- (a) Soil Name and Classification Symbol

The USC system is summarised in Figure E1.2.1. The primary division separates soil types on the basis of particle size into:

- Coarse grained soils - more than 50% of the material less than 60 mm is larger than 0.06 mm (60 µm).
- Fine grained soils - more than 50% of the material less than 60 mm is smaller than 0.06 mm (60 µm).

Initial classification is by particle size as shown in Table E1.2.1. Further classification of fine grained soils is based on plasticity.

TABLE E1.2.1 - CLASSIFICATION BY PARTICLE SIZE

NAME	SUB-DIVISION	SIZE
Clay (1)		< 2 µm
Silt (2)		2 µm to 60 µm
Sand	Fine Medium Coarse	60 µm to 200 µm 200 µm to 600 µm 600 µm to 2 mm
Gravel (3)	Fine Medium Coarse	2 mm to 6 mm 6 mm to 20 mm 20 mm to 60 mm
Cobbles (3)		60 mm to 200 mm
Boulders (3)		> 200 mm

Where a soil contains an appropriate amount of secondary material, the name includes each of the secondary components (greater than 12%) in increasing order of significance, eg sandy silty clay.

Minor components of a soil are included in the description by means of the terms "some" and "trace" as defined in Table E1.2.2.

TABLE E1.2.2 - MINOR SOIL COMPONENTS

TERM	DESCRIPTION	APPROXIMATE PROPORTION (%)
Trace	presence just detectable, little or no influence on soil properties	0-5
Some	presence easily detectable, little influence on soil properties	5-12

The USC group symbols should be included with each soil description as shown in Table E1.2.3

TABLE E1.2.3 - SOIL GROUP SYMBOLS

SOIL TYPE	PREFIX
Gravel	G
Sand	S
Silt	M
Clay	C
Organic	O
Peat	Pt

The group symbols are combined with qualifiers which indicate grading, plasticity or secondary components as shown on Table E1.2.4

TABLE E1.2.4 - SOIL GROUP QUALIFIERS

SUBGROUP	SUFFIX
Well graded	W
Poorly Graded	P
Silty	M
Clayey	C
Liquid Limit <50% - low to medium plasticity	L
Liquid Limit >50% - medium to high plasticity	H

(b) Grading

“Well graded”	Good representation of all particle sizes from the largest to the smallest.
“Poorly graded”	One or more intermediate sizes poorly represented
“Gap graded”	One or more intermediate sizes absent
“Uniformly graded”	Essentially single size material.

(c) Particle shape and texture

The shape and surface texture of the coarse grained particles should be described.

Angularity may be expressed as “rounded”, “sub-rounded”, “sub-angular” or “angular”.

Particle **form** can be “equidimensional”, “flat” or “elongate”.

Surface texture can be “glassy”, “smooth”, “rough”, “pitted” or “striated”.

(d) Colour

The colour of the soil should be described in the moist condition using simple terms such as:

Black	White	Grey	Red
Brown	Orange	Yellow	Green
Blue			

These may be modified as necessary by “light” or “dark”. Borderline colours may be described as a combination of two colours, eg red-brown.

For soils that contain more than one colour terms such as:

- Speckled Very small (<10 mm dia) patches
- Mottled Irregular
- Blotched Large irregular (>75 mm dia)
- Streaked Randomly oriented streaks

(e) Minor Components

Secondary and minor components should be individually described in a similar manner to the dominant component.

E1.3 Soil Condition

(a) Moisture

Soil moisture condition is described as “dry”, “moist” or “wet”.

The moisture categories are defined as:

Dry (D) - Little or no moisture evident. Soils are running. Moist (M) - Darkened in colour with cool feel. Granular soil particles tend to adhere. No free water evident upon remoulding of cohesive soils.

In addition the moisture content of cohesive soils can be estimated in relation to their liquid or plastic limit.

(b) Consistency

Estimates of the consistency of a clay or silt soil may be made from manual examination, hand penetrometer test, SPT results or from laboratory tests to determine undrained shear or unconfined compressive strengths. The classification of consistency is defined in Table E1.3.1.

TABLE E1.3.1 - CONSISTENCY OF FINE-GRAINED SOILS

TERM	UNCONFINED STRENGTH (kPa)	FIELD IDENTIFICATION
Very Soft	<25	Easily penetrated by fist. Sample exudes between fingers when squeezed in the fist.
Soft	25 - 50	Easily moulded in fingers. Easily penetrated 50 mm by thumb.
Firm	50 - 100	Can be moulded by strong pressure in the fingers. Penetrated only with great effort.
Stiff	100 - 200	Cannot be moulded in fingers. Indented by thumb but penetrated only with great effort.
Very Stiff	200 - 400	Very tough. Difficult to cut with knife. Readily indented with thumb nail.
Hard	>400	Brittle, can just be scratched with thumb nail. Tends to break into fragments.

Unconfined compressive strength as derived by a hand penetrometer can be taken as approximately double the undrained shear strength ($q_u = 2 c_u$).

(c) Density Index

The insitu density index of granular soils can be assessed from the results of SPT or cone penetrometer tests. Density index should not be estimated visually.

TABLE E1.3.2 - DENSITY OF GRANULAR SOILS

TERM	SPT N VALUE	STATIC CONE VALUE q_c (MPa)	DENSITY INDEX (%)
Very Loose	0 - 3	0 - 2	0 - 15
Loose	3 - 8	2 - 5	15 - 35
Medium Dense	8 - 25	5 - 15	35 - 65
Dense	25 - 42	15 - 20	65 - 85
Very Dense	>42	>20	>85

E1.4 Soil Structure

(a) Zoning

A sample may consist of several zones differing in colour, grain size or other properties. Terms to classify these zones are:

Layer - continuous across exposure or sample

Lens - discontinuous with lenticular shape

Pocket - irregular inclusion

Each zone should be described, their distinguishing features, and the nature of the interzone boundaries.

(b) Defects

Defects which are present in the sample can include:

- fissures
- roots (containing organic matter)
- tubes (hollow)
- casts (infilled)

Defects should be described giving details of dimensions and frequency. Fissure orientation, planarity, surface condition and infilling should be noted. If there is a tendency to break into blocks, block dimensions should be recorded

E1.5 Soil Origin

Information which may be interpretative but which may contribute to the usefulness of the material description should be included. The most common interpreted feature is the origin of the soil. The assessment of the probable origin is based on the soil material description, soil structure and its relationship to other soil and rock materials.

Common terms used are:

“Residual Soil” - Material which appears to have been derived by weathering from the underlying rock. There is no evidence of transport.

“Colluvium” - Material which appears to have been transported from its original location. The method of movement is usually the combination of gravity and erosion.

“Landslide Debris” - An extreme form of colluvium where the soil has been transported by mass movement. The material is obviously distributed and contains distinct defects related to the slope failure.

“Alluvium” - Material which has been transported essentially by water. usually associated with former stream activity.

“Fill” - Material which has been transported and placed by man. This can range from natural soils which have been

placed in a controlled manner in engineering construction to dumped waste material. A description of the constituents should include an assessment of the method of placement.

E1.6 Fine Grained Soils

The physical properties of fine grained soils are dominated by silts and clays.

The definition of clay and silt soils is governed by their Atterberg Limits. Clay soils are characterised by the properties of cohesion and plasticity with cohesion defines as the ability to deform without rupture. Silts exhibit cohesion but have low plasticity or are non-plastic.

The field characteristics of clay soils include:

- dry lumps have appreciable dry strength and cannot be powdered
- volume changes occur with moisture content variation
- feels smooth when moist with a greasy appearance when cut.

The field characteristics of silt soils include:

- dry lumps have negligible dry strength and can be powdered easily
- dilatancy - an increase in volume due to shearing - is indicated by the presence of a shiny film of water after a hand sample is shaken. The water disappears upon remoulding. Very fine grained sands may also exhibit dilatancy.
- low plasticity index
- feels gritty to the teeth

E1.7 Organic Soils

Organic soils are distinguished from other soils by their appreciable content of vegetable matter, usually derived from plant remains.

The soil usually has a distinctive smell and low bulk density.

The USC system uses the symbol Pt for partly decomposed organic material. The O symbol is combined with suffixes “O” or “H” depending on plasticity.

Where roots or root fibres are present their frequency and the depth to which they are encountered should be recorded. The presence of roots or root fibres does not necessarily mean the material is an “organic material” by classification.

Coal and lignite should be described as such and not simply as organic matter.

APPENDIX B – LABORATORY TEST RESULTS

STS Geotechnics Pty Ltd

14/1 Cowpasture Place, Wetherill Park NSW 2164

Phone: (02)9756 2166 | Email: enquiries@stsgео.com.au

**Shrink Swell Index Report**

Project: 2 - 8 GLEN AVENUE, NORTHMEAD

Client: NSW LAND & HOUSING CORPORATION

Address: Level 2, 31-39 Macquarie Street, Parramatta

Test Method: AS1289.7.1.1

Project No.: 30790/4406D-L

Report No.: 20/3400

Report Date: 1/10/2020

Page: 1 of 1

Sampling Procedure: AS 1289.1.3.1 Clause 3.1.3.2 - Thin Walled Sampler

STS / Sample No.		4406D-L/1	4406D-L/2	4406D-L/3			
Sample Location		Borehole 3 Refer to Drawing	Borehole 4 Refer to Drawing	Borehole 6 Refer to Drawing			
Material Description		Silty Sandy Clay, yellow brown, trace of gravel	Silty Sandy Clay, yellow brown/grey, trace of gravel	Silty Gravelly Clay, orange/yellow			
Depth (m)		0.5 - 0.7	0.5 - 0.7	0.4 - 0.6			
Sample Date		24/09/2020	24/09/2020	24/09/2020			
Shrink	Moisture Content (%)	19.5	22.3	26.4			
	Soil Crumbling	Nil	Nil	Nil			
	Extent of Cracking	Fine Cracks	Open Cracks	Open Cracks			
	Strain (%)	1.8	2.9	3.8			
Swell	Moisture Content Initial (%)	15.4	22.7	24.2			
	Moisture Content Final (%)	18.0	26.5	28.2			
	Strain (%)	0.0	0.0	0.1			
Inert Inclusions (%)		<5	<5	<25			
Shrink Swell Index (%)		1.0	1.6	2.1			

Remarks:



Accredited for compliance with ISO/IEC
17025 - Testing
The results of the tests, calibrations and/or
measurements included in this document are
traceable to Australian/national standards
NATA Accreditation Number 2750

Approved Signatory.....

Technician: DH

Orlando Mendoza - Laboratory Manager

CERTIFICATE OF ANALYSIS

Work Order : **ES2033875**
Client : **STS Geotechnics**
Contact : **ENQUIRES STS**
Address : Unit 14/1 Cowpasture Place
 Wetherill Park 2164
Telephone : ----
Project : 30055/30060/30790
Order number : E-2020-0380
C-O-C number : ----
Sampler : JK/MB
Site : ----
Quote number : EN/222
No. of samples received : 15
No. of samples analysed : 15

Page : 1 of 7
Laboratory : Environmental Division Sydney
Contact : Customer Services ES
Address : 277-289 Woodpark Road Smithfield NSW Australia 2164
Telephone : +61-2-8784 8555
Date Samples Received : 25-Sep-2020 10:30
Date Analysis Commenced : 28-Sep-2020
Issue Date : 07-Oct-2020 11:23



Accreditation No. 825
 Accredited for compliance with
 ISO/IEC 17025 - Testing

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted. This document shall not be reproduced, except in full.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results

Additional information pertinent to this report will be found in the following separate attachments: Quality Control Report, QA/QC Compliance Assessment to assist with Quality Review and Sample Receipt Notification.

Signatories

This document has been electronically signed by the authorized signatories below. Electronic signing is carried out in compliance with procedures specified in 21 CFR Part 11.

<i>Signatories</i>	<i>Position</i>	<i>Accreditation Category</i>
Ankit Joshi	Inorganic Chemist	Sydney Inorganics, Smithfield, NSW
Ivan Taylor	Analyst	Sydney Inorganics, Smithfield, NSW
Satishkumar Trivedi	Senior Acid Sulfate Soil Chemist	Brisbane Acid Sulphate Soils, Stafford, QLD



General Comments

The analytical procedures used by ALS have been developed from established internationally recognised procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are fully validated and are often at the client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

When sampling time information is not provided by the client, sampling dates are shown without a time component. In these instances, the time component has been assumed by the laboratory for processing purposes.

Where a result is required to meet compliance limits the associated uncertainty must be considered. Refer to the ALS Contact for details.

Key : CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.

LOR = Limit of reporting

^ = This result is computed from individual analyte detections at or above the level of reporting

Ø = ALS is not NATA accredited for these tests.

~ = Indicates an estimated value.

- ED045G: LOR raised for Chloride on sample 9 due to sample matrix.
- ASS: EA029 (SPOCAS): Laboratory determinations of ANC needs to be corroborated by effectiveness of the measured ANC in relation to incubation ANC. Unless corroborated, the results of ANC testing should be discounted when determining Net Acidity for comparison with action criteria, or for the determination of the acidity hazard and required liming amounts.
- ASS: EA029 (SPOCAS): Liming rate is calculated and reported on a dry weight basis assuming use of fine agricultural lime (CaCO_3) and using a safety factor of 1.5 to allow for non-homogeneous mixing and poor reactivity of lime. For conversion of Liming Rate from kg/t dry weight to kg/m³ in-situ soil, multiply reported results x wet bulk density of soil in t/m³.



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30055/6941	30055/6992	30055/6998	30060/1273	30060/1279
Client sampling date / time					24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00
Compound	CAS Number	LOR	Unit		ES2033875-001	ES2033875-002	ES2033875-003	ES2033875-004	ES2033875-005
				Result	Result	Result	Result	Result	Result
EA002: pH 1:5 (Soils)									
pH Value	----	0.1	pH Unit		7.3	5.6	7.1	6.0	8.1
EA010: Conductivity (1:5)									
Electrical Conductivity @ 25°C	----	1	µS/cm		161	144	453	249	242
EA055: Moisture Content (Dried @ 105-110°C)									
Moisture Content	----	0.1	%		7.7	9.8	11.4	9.8	7.9
ED040S : Soluble Sulfate by ICPAES									
Sulfate as SO4 2-	14808-79-8	10	mg/kg		40	20	240	40	80

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30790/S1	30790/S2	30790/S3	30790/S4	30790/ASS1
Client sampling date / time				24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00	
Compound	CAS Number	LOR	Unit	ES2033875-006	ES2033875-007	ES2033875-008	ES2033875-009	ES2033875-010	
				Result	Result	Result	Result	Result	
EA002: pH 1:5 (Soils)									
pH Value	----	0.1	pH Unit	6.2	5.2	7.6	7.4	----	
EA010: Conductivity (1:5)									
Electrical Conductivity @ 25°C	----	1	µS/cm	28	30	1970	33	----	
EA029-A: pH Measurements									
pH KCl (23A)	----	0.1	pH Unit	----	----	----	----	5.4	
pH OX (23B)	----	0.1	pH Unit	----	----	----	----	3.5	
EA029-B: Acidity Trail									
Titrateable Actual Acidity (23F)	----	2	mole H+ / t	----	----	----	----	9	
Titrateable Peroxide Acidity (23G)	----	2	mole H+ / t	----	----	----	----	98	
Titrateable Sulfidic Acidity (23H)	----	2	mole H+ / t	----	----	----	----	89	
sulfidic - Titrateable Actual Acidity (s-23F)	----	0.020	% pyrite S	----	----	----	----	<0.020	
sulfidic - Titrateable Peroxide Acidity (s-23G)	----	0.020	% pyrite S	----	----	----	----	0.157	
sulfidic - Titrateable Sulfidic Acidity (s-23H)	----	0.020	% pyrite S	----	----	----	----	0.143	
EA029-C: Sulfur Trail									
KCl Extractable Sulfur (23Ce)	----	0.020	% S	----	----	----	----	<0.020	
Peroxide Sulfur (23De)	----	0.020	% S	----	----	----	----	0.023	
Peroxide Oxidisable Sulfur (23E)	----	0.020	% S	----	----	----	----	0.023	
acidity - Peroxide Oxidisable Sulfur (a-23E)	----	10	mole H+ / t	----	----	----	----	14	
EA029-D: Calcium Values									
KCl Extractable Calcium (23Vh)	----	0.020	% Ca	----	----	----	----	0.072	
Peroxide Calcium (23Wh)	----	0.020	% Ca	----	----	----	----	0.073	
Acid Reacted Calcium (23X)	----	0.020	% Ca	----	----	----	----	<0.020	
acidity - Acid Reacted Calcium (a-23X)	----	10	mole H+ / t	----	----	----	----	<10	
sulfidic - Acid Reacted Calcium (s-23X)	----	0.020	% S	----	----	----	----	<0.020	
EA029-E: Magnesium Values									
KCl Extractable Magnesium (23Sm)	----	0.020	% Mg	----	----	----	----	<0.020	
Peroxide Magnesium (23Tm)	----	0.020	% Mg	----	----	----	----	<0.020	
Acid Reacted Magnesium (23U)	----	0.020	% Mg	----	----	----	----	<0.020	
Acidity - Acid Reacted Magnesium (a-23U)	----	10	mole H+ / t	----	----	----	----	<10	
sulfidic - Acid Reacted Magnesium (s-23U)	----	0.020	% S	----	----	----	----	<0.020	
EA029-H: Acid Base Accounting									



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30790/S1	30790/S2	30790/S3	30790/S4	30790/ASS1
Client sampling date / time					24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00
Compound	CAS Number	LOR	Unit		ES2033875-006	ES2033875-007	ES2033875-008	ES2033875-009	ES2033875-010
					Result	Result	Result	Result	Result
EA029-H: Acid Base Accounting - Continued									
ANC Fineness Factor	----	0.5	-	----	----	----	----	----	1.5
Net Acidity (sulfur units)	----	0.02	% S	----	----	----	----	----	0.04
Net Acidity (acidity units)	----	10	mole H+ / t	----	----	----	----	----	23
Liming Rate	----	1	kg CaCO3/t	----	----	----	----	----	2
Net Acidity excluding ANC (sulfur units)	----	0.02	% S	----	----	----	----	----	0.04
Net Acidity excluding ANC (acidity units)	----	10	mole H+ / t	----	----	----	----	----	23
Liming Rate excluding ANC	----	1	kg CaCO3/t	----	----	----	----	----	2
EA055: Moisture Content (Dried @ 105-110°C)									
Moisture Content	----	0.1	%		14.1	12.0	15.2	9.3	----
ED040S : Soluble Sulfate by ICPAES									
Sulfate as SO4 2-	14808-79-8	10	mg/kg		10	20	6790	<10	----
ED045G: Chloride by Discrete Analyser									
Chloride	16887-00-6	10	mg/kg		10	<10	<10	<100	----



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30790/ASS2	30790/ASS3	30790/ASS4	30790/ASS5	30790/ASS6
Client sampling date / time					24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00
Compound	CAS Number	LOR	Unit		ES2033875-011	ES2033875-012	ES2033875-013	ES2033875-014	ES2033875-015
					Result	Result	Result	Result	Result
EA029-A: pH Measurements									
pH KCl (23A)	----	0.1	pH Unit		4.7	4.9	7.9	4.9	4.2
pH OX (23B)	----	0.1	pH Unit		4.3	4.6	6.8	4.7	4.4
EA029-B: Acidity Trail									
Titrateable Actual Acidity (23F)	----	2	mole H+ / t		33	23	<2	23	83
Titrateable Peroxide Acidity (23G)	----	2	mole H+ / t		73	45	<2	48	110
Titrateable Sulfidic Acidity (23H)	----	2	mole H+ / t		40	22	<2	25	27
sulfidic - Titrateable Actual Acidity (s-23F)	----	0.020	% pyrite S		0.053	0.036	<0.020	0.037	0.133
sulfidic - Titrateable Peroxide Acidity (s-23G)	----	0.020	% pyrite S		0.117	0.072	<0.020	0.077	0.177
sulfidic - Titrateable Sulfidic Acidity (s-23H)	----	0.020	% pyrite S		0.064	0.036	<0.020	0.040	0.044
EA029-C: Sulfur Trail									
KCl Extractable Sulfur (23Ce)	----	0.020	% S		<0.020	0.036	0.295	0.033	<0.020
Peroxide Sulfur (23De)	----	0.020	% S		0.022	0.047	0.311	0.042	0.026
Peroxide Oxidisable Sulfur (23E)	----	0.020	% S		0.022	<0.020	<0.020	<0.020	0.026
acidity - Peroxide Oxidisable Sulfur (a-23E)	----	10	mole H+ / t		14	<10	10	<10	16
EA029-D: Calcium Values									
KCl Extractable Calcium (23Vh)	----	0.020	% Ca		0.032	<0.020	0.694	0.136	0.031
Peroxide Calcium (23Wh)	----	0.020	% Ca		0.030	0.022	0.743	0.140	0.033
Acid Reacted Calcium (23X)	----	0.020	% Ca		<0.020	0.022	0.048	<0.020	<0.020
acidity - Acid Reacted Calcium (a-23X)	----	10	mole H+ / t		<10	11	24	<10	<10
sulfidic - Acid Reacted Calcium (s-23X)	----	0.020	% S		<0.020	<0.020	0.039	<0.020	<0.020
EA029-E: Magnesium Values									
KCl Extractable Magnesium (23Sm)	----	0.020	% Mg		<0.020	<0.020	<0.020	0.042	0.034
Peroxide Magnesium (23Tm)	----	0.020	% Mg		<0.020	<0.020	0.021	0.043	0.035
Acid Reacted Magnesium (23U)	----	0.020	% Mg		<0.020	<0.020	0.021	<0.020	<0.020
Acidity - Acid Reacted Magnesium (a-23U)	----	10	mole H+ / t		<10	<10	18	<10	<10
sulfidic - Acid Reacted Magnesium (s-23U)	----	0.020	% S		<0.020	<0.020	0.028	<0.020	<0.020
EA029-F: Excess Acid Neutralising Capacity									
Excess Acid Neutralising Capacity (23Q)	----	0.020	% CaCO3		----	----	0.792	----	----
acidity - Excess Acid Neutralising Capacity (a-23Q)	----	10	mole H+ / t		----	----	158	----	----
sulfidic - Excess Acid Neutralising Capacity (s-23Q)	----	0.020	% S		----	----	0.253	----	----



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Client sample ID	30790/ASS2	30790/ASS3	30790/ASS4	30790/ASS5	30790/ASS6
Client sampling date / time					24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00	24-Sep-2020 00:00
Compound	CAS Number	LOR	Unit		ES2033875-011	ES2033875-012	ES2033875-013	ES2033875-014	ES2033875-015
					Result	Result	Result	Result	Result
EA029-F: Excess Acid Neutralising Capacity - Continued									
EA029-G: Retained Acidity									
HCl Extractable Sulfur (20Be)	----	0.020	% S		----	----	----	----	<0.020
Net Acid Soluble Sulfur (20Je)	----	0.020	% S		----	----	----	----	<0.020
acidity - Net Acid Soluble Sulfur (a-20J)	----	10	mole H+ / t		----	----	----	----	<10
sulfidic - Net Acid Soluble Sulfur (s-20J)	----	0.020	% pyrite S		----	----	----	----	<0.020
EA029-H: Acid Base Accounting									
ANC Fineness Factor	----	0.5	-		1.5	1.5	1.5	1.5	1.5
Net Acidity (sulfur units)	----	0.02	% S		0.07	0.05	<0.02	0.05	0.16
Net Acidity (acidity units)	----	10	mole H+ / t		47	30	<10	29	99
Liming Rate	----	1	kg CaCO3/t		4	2	<1	2	7
Net Acidity excluding ANC (sulfur units)	----	0.02	% S		0.07	0.05	<0.02	0.05	0.16
Net Acidity excluding ANC (acidity units)	----	10	mole H+ / t		47	30	10	29	99
Liming Rate excluding ANC	----	1	kg CaCO3/t		4	2	<1	2	7